The building at 120 The Terrace is not considered earthquake-prone under the current legislation.

This status comes as a result of an assessment process carried out under Council's previous earthquake-prone building policy 2009 which Council is reasonably satisfied qualifies as a previous assessment in terms of the current EPB Methodology. The original assessment process was part of a programme of assessments and subject to a moderation process and oversight by suitably qualified engineers.

As part of the previous assessment process an Initial Evaluation Procedure (IEP) was completed which indicated the building achieves 41%NBS.

The Initial Evaluation Procedure (IEP) assessment was a very basic and broad assessment carried out by engineers contracted to the Council. They visited the outside of the building to view the building in its environs and may have reviewed drawings held on file. Council initiated IEP's were carried out as a screening tool and should not be relied on by anyone for any other purpose and a detailed engineering inspection and/ or engineering calculations, may lead to a different result or seismic grade.

Ngā mihi
Kind Regards
Sharon Bennett
Resilience & Sustainability Team | Wellington City Council
M +64212278161 | E Sharon.Bennett@wcc.govt.nz | W Wellington.govt.nz |

Table IEP-1 Initial Evaluation Procedure Step 1

(Refer Table IEP - 2 for Step 2; Table IEP - 3 for Step 3, Table IEP - 4 for Steps 4, 5 and 6)

Building Name:	**120 The Terrace, Quest Wellingtonian**	**Ref.**	**WCC 4462 Lot#1**
Address:	**Wellington Central**	**By:**	**Beca - APG**
		Date:	**17/01/2007**

Step 1 - General Information

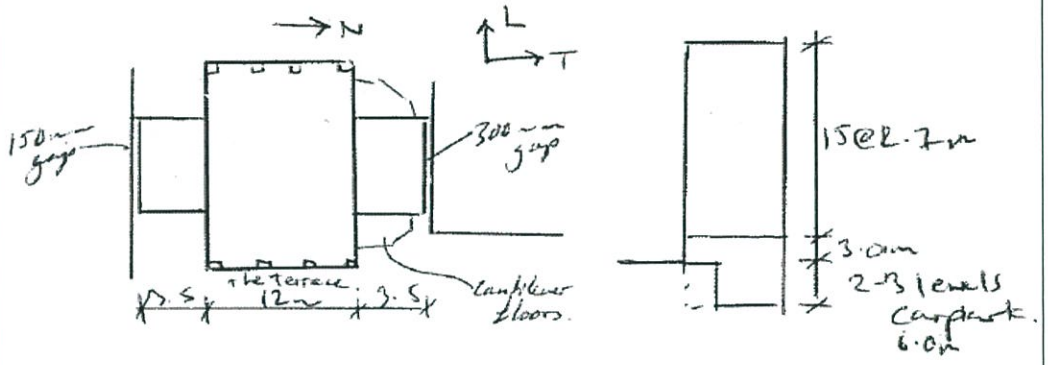
1.1 Photos (attach sufficient to describe building)

Front (bottom) - from The Terrace

<< Front (top) - from The Terrace

NOTE: THERE ARE MORE PHOTOS ON PAGE IEP-1a

1.2 Sketch of plan



1.3 List relevant features

1. The Quest Wellingtonian serviced appartments. Constructed in 1967.
2. RC frame.
3. Rectangular in plan with two wings (on both sides). Wings have RC walls. Carpark levels at rear.
4. 18-19 story high (16 storey above the terrace and 2-3 below). Slightly higher floor height at The Terrace level
5. Well maintained, no signs of significant deterioration observed
6. Potential pounding on each side in the longitudinal direction, buildings are similar heights, floors not aligned

1.4 Note information sources

	tick as appropriate
Visual Inspection of Exterior	☑
Visual Inspection of Interior	☐
Drawings (note type)	☐
Specifications	☐
Geotechical Reports	☐
Other (list)	☑

WCC Summary, Aerial photomap, Cityscope property datasheet

Table IEP-2 Initial Evaluation Procedure Step 2

(Refer Table IEP - 1 for Step 1; Table IEP - 3 for Step 3; Table IEP - 4 for Steps 4, 5 and 6)

Building Name:	120 The Terrace, Quest Wellingtonian	**Ref.**	WCC 4462 Lot#1
Address:	Wellington Central	**By**	Beca - APG
Direction Considered:	**a) Longitudinal b) Transverse**		
(Choose worse case if clear at start. Complete IEP-2 and IEP-3 for each if in doubt)		**Date:**	17/01/2007

Step 2 - Determination of $(\%NBS)_b$

2.1 Determine nominal (%NBS) = $(\%NBS)_{nom}$

(Baseline (%NBS) for particular building - refer Section B5)

a) Date of Design and Seismic Zone

Date of Design: ○ Pre 1935 — See Note 1, 3
○ 1935-1965
◉ 1965-1976
○ 1976-1992 — See Note 2
○ 1992-2004

Building Category: Others

Seismic Zone: Zone A

b) Soil Type

From NZS1170.5:2004, Cl 3.1.3 :

NZS1170.5:2004
- ☑ A or B Rock
- ☐ C Shallow Soil
- ☐ D Soft Soil
- ☐ E Very Soft Soil

From NZS4203:1992, Cl 4.6.2.2 : (for 1992 to 2004 only and only if known)

NZS4203:1992
- ☑ Rigid
- ☐ Intermediate or Not Known

c) Estimate Period, *T*

L-direction H= 50m, T-direction H=47m

			Longitudinal	Transverse
		h_n =	47	50 m
		A_c =	0.00	0.00 m^2
Moment Resisting Concrete Frames:	$T = 0.09h_n^{0.75}$		☑ MRCF	☑ MRCF
Moment Resisting Steel Frames:	$T = 0.14h_n^{0.75}$		☐ MRSF	☐ MRSF
Eccentrically Braced Steel Frames:	$T = 0.08h_n^{0.75}$		☐ EBSF	☐ EBSF
All Other Frame Structures:	$T = 0.06h_n^{0.75}$		☐ Others	☐ Others
Concrete Shear Walls	$T = 0.09h_n^{0.75}/\sqrt{A_c}$		☐ CSW	☐ CSW
Masonry Shear Walls:	$T \leq 0.4$sec		☐ MSW	☐ MSW
User Defined (input Period):			☐ Defined	☐ Defined

Where h_n = height in m from the base of the structure to the uppermost seismic weight or mass.

$A_c = \Sigma A_i(0.2 + L_{wi}/h_n)^2$

A_i = cross-sectional shear area of shear wall i in the first storey of the building, in m^2

l_{wi} = length of shear wall i in the first storey in the direction parallel to the applied forces, in m with the restriction that l_{wi}/h_n shall not exceed 0.9

Longitudinal	Transverse	
1.62	1.69	Seconds

d) $(\%NBS)_{nom}$ determined from Figure 3.3

Longitudinal: 13.82 $(\%NBS)_{nom}$
Transverse: 14.48 $(\%NBS)_{nom}$

Note 1: For buildings designed prior to 1965 and known to be designed as public buildings in accordance with the code of the time, multipy $(\%NBS)_{nom}$ by 1.25.
For buildings designed 1965 - 1976 and known to be designed as public buildings in accordance with the code of the time, mulitply $(\%NBS)_{nom}$ by 1.33 - Zone A, or by 1.2 - Zone B

FALSE

Note 2: For reinforced concrete buildings designed between 1976-84 multiply $(\%NBS)_{nom}$ by 1.2

FALSE

Note 3: For buildings designed prior to 1935 multiply $(\%NBS)_{nom}$ by 0.8 except for Wellington where the factor may be taken as 1.

FALSE

Longitudinal: 13.82 $(\%NBS)_{nom}$
Transverse: 14.48 $(\%NBS)_{nom}$
(Scaled as per Notes 1 to 3)

Contiued over page.....

2.2 Near Fault Scaling Factor, Factor A

If $T \leq 1.5$sec, Factor A = 1

a) **Near Fault Factor, *N(T,D)*** (from NZS1170.5:2004, Cl 3.1.6)
Longitudinal: 1.03
Transverse: 1.05

b) **Near Fault Scaling Factor** = 1/N(T,D)

Factor A
Longitudinal: 0.97
Transverse: 0.95

2.3 Hazard Scaling Factor, Factor B

a) **Hazard Factor, *Z, for site*** (from NZS1170.5:2004, Table 3.3)
Site Area : Wellington CBD (north of Basin
Z = 0.4
Z_{1992} =

b) **Hazard Scaling Factor**
For pre 1992 = $1/Z$
For 1992 onwards = Z_{1992}/Z

(Where Z_{1992} is the NZS4203:1992 Zone Factor from accompanying Figure 3.5(b))

Factor B 2.50

2.4 Return Period Scaling Factor, Factor C

a) **Building Importance Level** (from NZS1170.0:2004, Table 3.1 and 3.2)
Choose Importance Level
☐ 1 ☉ 2 ☐ 3 ☐ 4

b) **Return Period Scaling Factor from accompanying Table 3.1**

Factor C 1.00

2.5 Ductility Scaling Factor, D

a) **Assessed Ductility of Existing Structure, μ** (shall be less than maximum given in accompanying Table 3.2)
μ = 2.00 Longitudinal Direction
μ = 2.00 Transverse Direction
max = 2

b) **Ductility Scaling Factor**

		Longitudinal	Transverse
For pre 1976	=	k_μ	k_μ
	=	2.00	2.00
For 1976 onwards	=	1	1

(where k_μ is NZS1170.5:2004 Ductility Factor, from accompanying Table 3.3)

Factor D
Longitudinal: 2.00
Transverse: 2.00

2.6 Structural Performance Scaling Factor, Factor E

a) **Structural Performance Factor, S_p from accompanying Figure 3.4** 0.70

b) **Structural Performance Scaling Factor** = $1/S_p$

Factor E 1.43

2.7 Baseline %NBS for Building, $(\%NBS)_b$

(equals $(\%NSB)_{nom}$ x A x B x C x D x E)

Longitudinal : 95.84

Transverse : 98.53

Table IEP-3 Initial Evaluation Procedure Step 3

(Refer Table IEP - 1 for Step 1; Table IEP - 2 for Step 2; Table IEP - 4 for Steps 4, 5 and 6)

Building Name:	120 The Terrace, Quest Wellingtonian	**Ref.**	WCC 4462 Lot#1
Address:	Wellington Central	**By:**	Beca - APG
Direction Considered:	**a) Longitudinal & b) Transverse**		
(Choose worse case if clear at start. Complete IEP-2 and IEP-3 for each if in doubt)		**Date:**	17/01/2007

a) Longitudinal Direction

Step 3 - Assessment of Performance Achievement Ratio (PAR)

(Refer Appendix B - Section B3.2)

Critical Structural Weakness	Effect on Structural Performance (Choose a value - Do not interpolate)	Building Score
3.1 Plan Irregularity	☐ Severe ☐ Significant ☑ Insignificant	Factor A: 1.0
Comment		
3.2 Vertical Irregularity	☐ Severe ☑ Significant ☐ Insignificant	Factor B: 0.7
Comment	**Higher first storey by 10-15% therefore significant soft storey effect**	
3.3 Short Columns	☐ Severe ☐ Significant ☑ Insignificant	Factor C: 1.0
Comment		

3.4 Pounding Potential

(Estimate D1 and D2 and set D = the lower of the two, or =1.0 if no potential for pounding)

☐ ***Potential for pounding***
☑ ***No potential for pounding***

a) Factor D1: - Pounding Effect
Select appropriate value from Table

Note:
Values given assume the building has a frame structure. For stiff buildings (eg with shear walls), the effect of pounding may be reduced by taking the co-efficient to the right of the value applicable to frame buildings.

Factor D1 For Longitudinal Direction: 1

Table for Selection of Factor D1	Severe	Significant	Insignificant
Separation	0<Sep<.005H	.005<Sep<.01H	Sep>.01H
Alignment of Floors within 20% of Storey Height	☐ 0.7	☐ 0.8	☑ 1
Alignment of Floors not within 20% of Storey Heigh	☐ 0.4	☐ 0.7	☐ 0.8

Comment: **Freestanding**

b) Factor D2: - Height Difference Effect
Select appropriate value from Table

Factor D2 For Longitudinal Direction: 1

Table for Selection of Factor D2	Severe 0<Sep<.005H	Significant .005<Sep<.01H	Insignificant Sep>.01H
Height Difference > 4 Storeys	☐ 0.4	☐ 0.7	☑ 1
Height Difference 2 to 4 Storeys	☐ 0.7	☐ 0.9	☐ 1
Height Difference < 2 Storeys	☐ 1	☐ 1	☐ 1

Comment: **Freestanding**

Factor D For Longitudinal Direction: 1.0

(Set D = lesser of D1 and D2 or..
set D = 1.0 if no prospect of pounding)

3.5 Site Characteristics - *(Stability, landslide threat, liquefaction etc)*

Severe	Significant	Insignificant
1 ☐ 0.5max	☐ 0.7	☑ 1

Comment:

3.6 Other Factors

Factor F: 1.5 — **For ≤ 3 storeys - Maximum value 2.5, otherwise - Maximum value 1.5. No minimum.**

Record rationale for choice of Factor F:

Well maintained, likely good diaphragm, likely to achieve higher ductility than 2.

3.7 Performance Achievement Ratio (PAR) (equals **A x B x C x D x E x F**) **1.05**

b) Transverse Direction

Step 3 - Assessment of Performance Achievement Ratio (PAR)

(Refer Appendix B - Section B3.2)

Critical Structural Weakness	Effect on Structural Performance (Choose a value - Do not interpolate)	Building Score

3.1 Plan Irregularity

Effect on Structural Performance ☐ Severve ☐ Significant ☑ Insignificant — Factor A: 1.0

Comment

3.2 Vertical Irregularity

Effect on Structural Performance ☐ Severve ☑ Significant ☐ Insignificant — Factor B: 0.7

Comment Higher first storey by 10-15% therefore significant soft storey effect

3.3 Short Columns

Effect on Structural Performance ☐ Severve ☐ Significant ☑ Insignificant — Factor C: 1.0

Comment Possible short columns but RC walls will prevent significant effect.

3.4 Pounding Potential

(Estimate D1 and D2 and set D = the lower of the two, or =1.0 if no potential for pounding)

☑ *Potential for pounding*

☐ *No potential for pounding*

a) Factor D1: - Pounding Effect

Select appropriate value from Table

Note:
Values given assume the building has a frame structure. For stiff buildings (eg with shear walls), the effect of pounding may be reduced by taking the co-efficient to the right *of the value applicable to frame buildings.*

Factor D1 For Transverse Direction: 0.4

Table for Selection of Factor D1	Severe	Significant	Insignificant
Separation	0<Sep<.005H	.005<Sep<.01H	Sep>.01H
Alignment of Floors within 20% of Storey Height	☐ 0.7	☐ 0.8	☐ 1
Alignment of Floors not within 20% of Storey Height	☑ 0.4	☐ 0.7	☐ 0.8

Comment: Gap=150mm/44m=0.0034<0.005H, floors not aligned within 20%

b) Factor D2: - Height Difference Effect

Select appropriate value from Table

Factor D2 For Transverse Direction: 1

Table for Selection of Factor D2	Severe	Significant	Insignificant
	0<Sep<.005H	.005<Sep<.01H	Sep>.01H
Height Difference > 4 Storeys	☐ 0.4	☐ 0.7	☐ 1
Height Difference 2 to 4 Storeys	☐ 0.7	☐ 0.9	☐ 1
Height Difference < 2 Storeys	☑ 1	☐ 1	☐ 1

Comment: Buildings similar height.

Factor D For Transverse Direction: 0.4

(Set D = lesser of D1 and D2 or.. set D = 1.0 if no prospect of pounding)

3.5 Site Characteristics - *(Stability, landslide threat, liquefaction etc)*

	Severe	Significant	Insignificant
1	☐ 0.5max	☐ 0.7	☑ 1

Comment:

3.6 Other Factors

Factor F: 1.5 — For ≤ 3 storeys - Maximum value 2.5, otherwise - Maximum value 1.5. No minimum.

Record rationale for choice of Factor F:

Well maintained, likely good diaphragm, likely to achieve higher ductility than 2. Pounding is too conservative if transverse walls are present.

3.7 Performance Achievement Ratio (PAR) (equals A x B x C x D x E x F) — Transverse: 0.42

Table IEP- 4 Initial Evaluation Procedure Steps 4, 5 and 6

(Refer Table IEP - 1 for Step 1; Table IEP - 2 for Step 2; Table IEP - 3 for Step 3)

Building Name:	120 The Terrace, Quest Wellingtonian	**Ref.**	WCC 4462 Lot#1
Address:	Wellington Central	**By:**	Beca - APG
		Date:	17/01/2007

Step 4 - Percentage of New Building Standard (%NBS)

	Longitudinal	Transverse
4.1 Assessed Baseline ($\%NBS)_b$ (from Table IEP - 1)	95.8%	98.5%
4.2 Performance Achievement Ratio (PAR) (from Table IEP - 2)	1.05	0.42
4.3 PAR x Baseline (%NBS)b	101%	41%
4.4 Percentage New Building Standard (%NBS) (Use lower of two values from Step 3.3)		41%

Step 5 - Potentially Earthquake Prone?

(Mark as appropriate)

%NBS ≤ 33	NO

Step 6 - Potentially Earthquake Risk?

(Mark as appropriate)

%NBS < 67	YES

Step 7 - Provisional Grading for Seismic Risk based on IEP

Seismic Grade	C

Evaluation Confirmed by Beca **Signature**

On behalf of WCC **Name**

CPEng. No

Relationship between Grade and SPS:

Grade:	A+	A	B	C	D	E
%NBS:	> 100	100 to 80	80 to 67	67 to 33	33 to 20	< 20

Table IEP-1a Additional Photos and Sketches

(Refer Table IEP - 2 for Step 2; Table IEP - 3 for Step 3, Table IEP - 4 for Steps 4, 5 and 6)

Street Number & Name:	120 The Terrace, Quest Wellingtonian	**Ref.**	WCC 4462 Lot#1
Location:	Wellington Central	**By:**	Beca - APG
		Date:	17/01/2007

Add any additional photographs, notes or sketches required below:

Note: print this page separately

Rear

Rear (bottom)

Clendon Burns & Park Ltd
CONSULTING CIVIL & STRUCTURAL ENGINEERS

LEVEL 1 RAPID SEISMIC ASSESSMENT REPORT

JOB:	120 The Terrace, Wellington	**JOB NO:**	216233
TO:	Your Property Matters	**ATTENTION:**	Carl Coetzee
INSPECTOR:	Anthony Taylor & Cale Wood		
INSPECTION DATE:	14/11/16	**ISSUE DATE**	25/11/16

This Rapid Seismic Assessment Report has been prepared at the request of the Your Property Matters (YPM) to be used for their purposes only, and neither Clendon Burns & Park Ltd nor any of its employees accept any responsibility on any ground whatsoever to any other party or person who relies upon it.

The report is based only on a visual walk-through inspection of the buildings unless noted otherwise below. No destructive tests or invasive investigations have been undertaken.

This report is limited to inspection of the building structure only, it does not cover other services or other aspects of the New Zealand Building Code compliance including water tightness envelope to the building.

Our inspection was carried out at the date and time noted on the attached Rapid Assessment form. If there has been any significant seismic activity since then, the building's status may have changed and a re-inspection should be carried out.

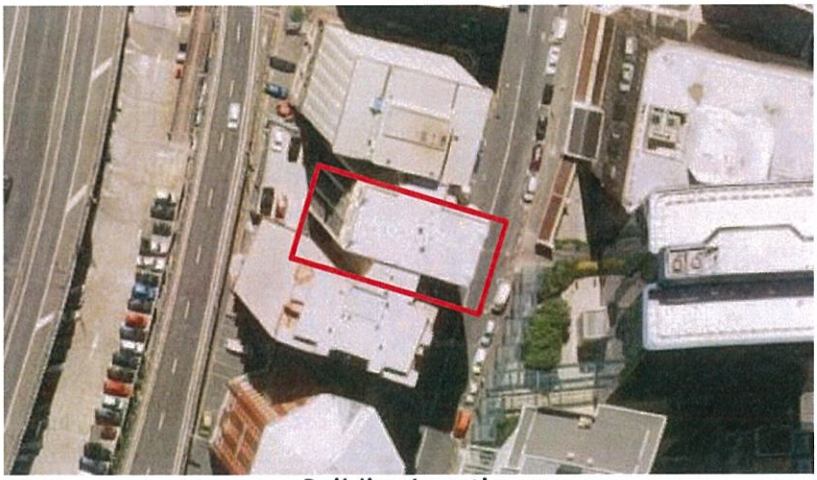
Any decision made to re-occupy the building should be based on the findings of this report together with any prior knowledge of the seismic risk of the building and any relevant legislative obligations such as the Health and Safety in Employment Act.

Purpose of Inspection:

Anthony Taylor and Cale Wood of Clendon Burns & Park Ltd completed a level 1 inspections of the structural damage to the concrete shear wall building located at 120 The Terrace. Anthony and Cale were advised by a member of staff who had previously been through the building and found very little evidence of damage.

Your Building Matters requested an inspection to identify possible structural damage to building, which is located at 120 The Terrace, Wellington, following the 14th November 2016 magnitude 7.8 earthquake centred 15 kilometres north-east of Culverden in the Canterbury region, referred to as the Kaikoura Earthquake.

Clendon Burns & Park Ltd was also previously engaged to carry out post-earthquake rapid assessments following both the 21st July 2013 Cook Strait and 16th August 2013 Lake Grassmere earthquakes.



Building Location

Clendon Burns & Park Ltd
p +64 4 472 1412 f +64 4 472 1417
e cbp@cbp.co.nz w www.cbp.co.nz
a 15a Everton Terrace P O Box 10 348 Wellington New Zealand

Building Street View

Extent of Inspection:

This structural inspection has been undertaken generally in accordance with the Level 1 Rapid Assessment method that was established from the Canterbury Earthquakes and in the New Zealand National Society of Earthquake Engineering (NZSEE) Guidelines for Building Safety Evaluations.

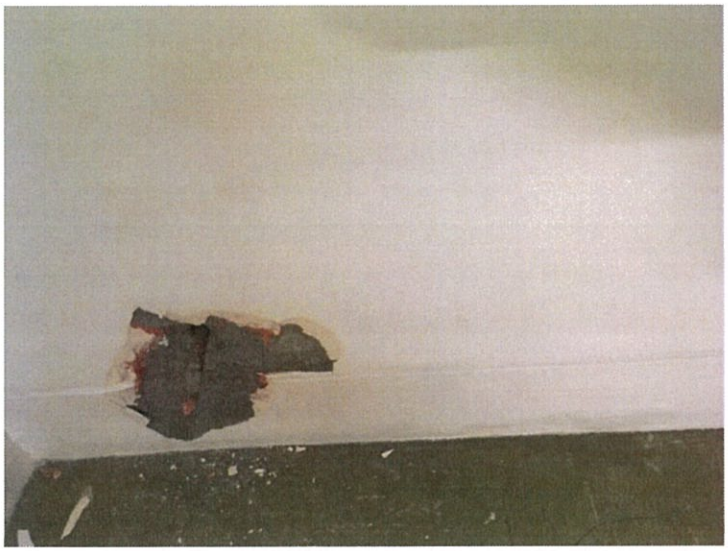
It should be noted that not all rooms or surfaces were inspected. Inspection was undertaken to the external areas front and from the rear carpark deck, and to the common stairwell and hallway areas.

General:

The year the building was constructed is unknown. The building main lateral load resisting system seems to consist of reinforced concrete shear walls. The building consists of 11 levels above ground and a single sub-ground level basement.

A blow out of plaster where a steel rod had pierce through was observed, it was deemed that this was pre-existing and had just been pushed through more. Also some damage was observed to the rear corner of the stair core which seems to have pounded with the adjacent building. This was difficult to observe in detail the extent of the damage.

Additional photos of items of concern:



Steel connection rod penetrating through concrete wall

Clendon Burns & Park Ltd
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e cbp@cbp.co.nz w www.cbp.co.nz
a 15a Everton Terrace P O Box 10 348 Wellington New Zealand

Damage to the corner of the building from pounding with adjacent building

Conclusions:

A structural inspection was undertaken following the 14th November 2016 magnitude 7.8 earthquake centred near Culverden. No structural damage was observed to the primary structure of the reinforced concrete building.

Based on our observations the seismic strength of the building has not altered as a result of this earthquake. Hence the risk remains the same as it was prior to the seismic event on the 14th November 2016.

Report Prepared by:
CLENDON BURNS & PARK LTD

Cale Wood BE(Hons) GIPENZ
DESIGN ENGINEER

Reviewed By:
CLENDON BURNS & PARK LTD

A G Taylor BE (Hons), MIPENZ, CPEng, IntPE
ASSOCIATE DIRECTOR
CPEng No. 228777

EARTHQUAKE RAPID ASSESSMENT FORM

Complex Residential and all Non-Residential Buildings Level 1

ASSESSMENT

Fields with asterisks (*) are mandatory, others are optional.

1 Assessor Name* Anthony Taylor

Assessor ID* 228777 CPEng Authority*

2 Assessment Date* 14 / 11 / 16 (Day / Month / Year) Assessment Time* 10 00 (Hour / Minute, *to nearest half hour*) A ✓ AM B ○ PM

BUILDING IDENTIFICATION

3 Building Name

Unit / Number* / 120

Street* The Terrace

City/Town* Wellington

GPS (Degree with 5 decimals after comma) South -, East ,

Other ID or access Photo taken A ○ No B ○ Yes Photo ID.

4 Contact Name

Type A ○ Owner B ○ Tenant C ○ Other

Phone (with area code) (0)

5 Existing Placard* ○ None ○ W ○ Y1 ○ Y2 ○ R1 ○ R2 Date* (Day / Month / Year) Team ID*

BUILDING DESCRIPTION

6

Dimensions	Constr. Age	Building Type	Structure Type	Cladding Type
Storeys above ground incl. ground floor: 1 1	A ○ <1935	A ✓ Complex residential	A ○ Timber frame	A ○ Brick veneer
Storeys below ground: 1	B ○ 1935-1976	B ○ School	B ○ Steel frame	B ○ Concrete panels
Footprint (m²):	C ○ 1977-1984	C ○ Commercial/Office	C ○ Concrete frame	C ○ Steel
	D ○ 1985-2000	D ○ Industrial	D ✓ Concrete shear wall	D ○ Glass
	E ○ >2000	E ○ Critical facility	E ○ Tilt-up concrete	E ○ Lightweight
	F ✓ Unknown	F ○ Public assembly	F ○ Reinforced masonry	F ○ Other:
		G ○ Other:	G ○ Unreinforced masonry	
			H ○ Other:	

EXTERNAL RISKS

7

Potential Cause*	A Yes	B No
1 Objects falling from adjacent buildings. Adjacent building ID or address:	○	✓
2 Land instability above	○	✓
3 Land instability below	○	✓
4 Other	○	○

If required add sketch on separate page showing extent and nature of the external risk factors.

VERSION 01 – APRIL 2014

OBSERVED DAMAGE

8

Structural Component*	Observed Condition: N/A	Unknown	Minor or None	Moderate	Severe
	N/A	A	B	C	D
1 Collapse, partial collapse, off foundation	○	○	○	○	○
2 Building or storey leaning	○	○	○	○	○
3 Structural damage to vertical system	○	○	✓	○	○
4 Structural damage to lateral system	○	○	✓	○	○
5 Chimney, parapet or other falling hazard	○	○	○	○	○
5 Ground slope movement or cracking	○	○	○	○	○
6 Other:	○	○	○	○	○

Comments:

9 Estimated Damage A ○ None B ✓ 0-10% C ○ 11-30% D ○ 31-60% E ○ 61-100%

SUGGESTED FURTHER ACTIONS

10

Recommended further Assessment*	Safety Cordon*	Barricades*	Urgency of suggested action*
A ✓ None B ○ Level 2 Rapid Assessment (tick below if particular expertise is required) B1 ○ Structural Engineer B2 ○ Geotechnical Engineer B3 ○ Other: C ○ Further evaluation to be arranged by building owner:	A ✓ None required B ○ Cordon required Describe extent (add diagram on separate sheet if required)	A ✓ None required B ○ Barricades already in place C ○ Barricades required Describe extent (add diagram on seperate sheet if required)	A ✓ Standard B ○ Immediate action required

SUMMARY

11

Observed Damage	Level 1 Rapid Assessment Outcome*
Light or no damage	W ✓ CAN BE USED (From assessment no known dangers)
Moderate damage	Y1 ○ RESTRICTED ACCESS TO PART(S) OF THE BUILDING ONLY
	Y2 ○ RESTRICTED ACCESS – SHORT TERM ENTRY ONLY with or without supervision Access to be supervised A ○ Yes B ○ No
Heavy damage	R1 ○ ENTRY PROHIBITED (At risk from external factors)
	R2 ○ ENTRY PROHIBITED (Severe damage to building)

12

Survey Extent*		
Exterior	A	✓ Partial
	B	○ Complete
Interior	C	○ Not accessed
	D	✓ Partial
	E	○ Complete

Assessor Signature* [signature] CPEng No 303598

NOTES

13 No individual Apartments were assessed

If required add a sketch on a separate sheet of paper showing building damage, access restrictions or cordoning areas. Identify the building on the sketch and staple the sheet to this assessment form.

Sketch included on separate page? ○ Yes ○ No